# BOND STRENGTH OF CORRODED HIGH YIELD STEEL BARS EMBEDDED IN NORMAL STRENGTH CONCRETE

# THOMAS TAN

PERPUSTAKAAN UNUVERSITI MALAYSIA SABAH

# THESIS SUBMITTED IN FULFILLMENT FOR THE DEGREE OF MASTER OF ENGINEERING

# FACULTY OF ENGINEERING UNIVERSITY MALAYSIA SABAH 2019



#### UNIVERSITI MALAYSIA SABAH

BORANG PENGESAHAN STATUS TESIS

# JUDUL:BOND STRENGTH OF CORRODED HIGH YIELD STEEL BARS<br/>EMBEDDED IN NORMAL STRENGTH CONCRETE

#### LIAZAH: SARJANA KEJURUTERAAN (KEJURUTERAAN AWAM)

Saya **THOMAS TAN**, Sesi **2015-2019**, mengaku membenarkan tesis Sarjana ini disimpan di Perpustakaan Universiti Malaysia Sabah dengan syarat-syarat kegunaan seperti berikut: -

- 1. Tesis ini adalah hak milik Universiti Malaysia Sabah.
- 2. Perpustakaan Universiti Malaysia Sabah dibenarkan membuat salinan untuk tujuan pengajian sahaja.
- 3. Perpustakaan dibenarkan membuat salinan tesis sebagai bahan pertukaran antara institusi pengajian tinggi.
- 4. Sila tandakan ( / ):



(Prof. Ir. Dr. Abdul Karim Mirasa) Penyelia Utama

(Dr. Ahmad Nurfaidhi Rizalman) Penyelia Bersama



: 10<sup>th</sup> September 2019 Tarikh

### DECLARATION

I declare that this thesis submitted to the University Malaysia Sabah, in fulfillment for the degree of Master of Engineering (MEng) in Civil Engineering. This thesis has not been submitted, to any others university for any degrees. I also certify that this works described herein entirely is my own, except for equations, quotations and references as the sources have been duly acknowledged and this thesis may be available within the university library and can be photocopied or loaned to other libraries for purposes of consultation.

10<sup>th</sup> September 2019

Thomas THOMAS TAN

MK1521020T



#### CERTIFICATION

- NAME : THOMAS TAN
- MATRIX NO : MK1521020T
- TITLE: BOND STRENGTH OF CORRODED HIGH YIELD STEELBARS EMBEDDED IN NORMAL STRENGTH CONCRETE
- DEGREE : MASTER OF ENGINEERING (CIVIL ENGINEERING)
- VIVA-VOCE : 25<sup>th</sup> JULY 2019

#### **CERTIFIED BY:**

**1. MAIN SUPERVISOR** 

Professor Ir Dr Abdul Karim bin Mirasa

Signature

#### 2. CO-SUPERVISOR

Dr Ahmad Nurfaidhi bin Rizalman

Signature



### ACKNOWLEDGEMENT

I would like to express my deepest gratitude to my main supervisor Professor Ir. Dr. Abdul Karim bin Mirasa without his expertise and his guidance are provided, I would not have been able to complete this study. I would like to thanks to my co-supervisor Dr. Ahmad Nurfaidhi bin Rizalman for his insight, expertise, and valuable comments while reviewing this thesis.

I am so gratefully and would like to acknowledge the generous support I received from the University Malaysia Sabah and the Government of Public Works Department Sabah for their assistance in completed my study in their Laboratory, without such support I could not complete this thesis: Professor Ir. Dr. Abdul Karim bin Mirasa of University Malaysia Sabah, Faculty Engineering (Dean) and Datuk Ir Hj. Amrullah bin Hj. Kamal (Director) and Mr. Vyner Sikodol (Chief Assistant Director) of Public Works Department Sabah.

I would like to express my appreciations to Mr. Ismail bin Usop (Assistant Engineer) of Public Works Department Sabah, Material Testing Laboratory Kota Kinabalu and staffs of University Malaysia Sabah, for their advice and technical support throughout my experiments program. In addition, my deepest appreciations to Mr. Chia Min Kong (Contractor), Mr. Jamealy bin Abdullah (Contractor), Mr. Vun Soo Kong, Director of Pemborong Seri Jindo and Mr. Tan Kia Hong, Operation Manager of Kontraktor Fokus Niaga Sdn. Bhd. who helped throughout the completion of this works both physically and technically with my analytical works and emotions throughout the whole process.

Finally, I would like to give my deepest heartfelt thanks to Madam Angielina David, Miss Vivilen Langsi, Miss Stella Myfey John, Miss Danielle Myfey John, and my family for their constants support and encouragement.

THOMAS TAN 10<sup>th</sup> September 2019



### ABSTRACT

Strength and performance of reinforced-concrete depend on the good bond strength, between high yield steel bars and concrete, and the effects of corrosion on high yield steel bars in a range of 12 mm, 16 mm and 25 mm diameter reinforced the concrete for its bond strength and are debated endlessly on-site. To investigate the effects of corrosion on the bond strength between the high yield steel bars and concrete, 36 specimens of tensile test, 72 specimens of pullout test and 12 specimens of flexural test conducted. Descriptions of specimen data taken an average of three reading for tensile test and pullout test except for flexural test, uncorroded as control specimen and corroded as a final specimen. High yield steel bars ranges from 12 mm, 16 mm and 25 mm diameter 600 mm long used for tensile test, high yield steel bars ranges 12 mm, 16 mm and 25 mm diameter 800 mm long used as reinforcement bars and embedded vertically in a concrete mold 150 mm x 150 mm x 150 mm concrete grade 20 and concrete grade 30 for pullout test and 1500 mm x 150 mm x 200 mm beam concrete grade 20 for the flexural test. Results show, weight of high yield steel bars reduced by 0.13%, 0.07% and 0.02%, area reduced by 0.13%, 0.07% and 0.02% on 8 months and 12 months reduced by 0.13%, 0.04%, and 0.03%, the area reduced by 0.12%, 0.04% and 0.03% for high yield steel bars 12 mm, 16 mm and 25 mm diameter for the tensile test. The weight of high yield steel bars reduced by 0.14%, 0.05%, and 0.02%, area reduced by 0.13%, 0.06%, and 0.02% on 8 months and 12 months for high yield steel bars 12 mm, 16 mm and 25 mm diameter for pullout test concrete grade 20 and concrete 30. Weight of high yield steel bars reduced by 0.08%, 0.05% and 0.01%, area reduced by 0.08%, 0.04% and 0.01% for 8 months and 12 months for the high yield steel bars 12 mm, 16 mm and 25 mm diameter for flexural test concrete grade 20. Weight and area of high yield steel bar 12 mm, 16 mm and 25 mm diameter unchanged for the 4 months for pullout test concrete grade 20 and concrete grade 30, and flexural test concrete grade 20. In sum, the researcher discovered the uncleaned corrosion on high yield steel bars surface influences the bond strength on 4 months of corrosion and the cleaned corrosion on the high yield steel bars surface did not influence the bond strength on the 8 months and 12 months of corrosion for pullout test and flexural test conducted.



#### ABSTRAK

## KEKUATAN IKATAN BESI WAJA BERKARAT DI TANAM DALAM KEKUATAN KONKRIT BIASA

Kekuatan dan prestasi konkrit bertetulang bergantung kepada kekuatan ikatan yang sempurna antara besi waja dan konkrit. Kesan karat pada besi waja bersaiz 12 mm, 16 mm, 25 mm untuk tetulang pada konkrit selalu menjadi perbahasan ditapak bina. Penyiasatan terhadap kesan karat pada kekuatan besi waja dan konkrit bertetulang, sebanyak 36 spesimen untuk ujian regangan, 72 spesimen untuk ujian tarik keluar dan 12 spesimen untuk ujian lenturan telah dilaksanakan. Keterangan data daripada spesimen diambil berdasarkan purata daripada tiga bacaan untuk ujian regangan dan ujian tarik keluar kecuali ujian lenturan. Besi waja tidak berkarat digunakan sebagai spesimen rujukan ujian, manakala besi waja berkarat diuji untuk dapatan perbezaan kekuatan ikatan antara besi waja dan konkrit untuk diperbandingkan. Besi waja saiz 12 mm, 16 mm dan 25 mm 600 mm panjang digunakan untuk ujian regangan. Besi waja saiz 12 mm, 16 mm dan 25 mm 800 mm panjang digunakan sebagai tetulang konkrit dengan kaedah diletak menegak dalam acuan 150 mm x 150 mm x 150 mm berkonkritkan gred 20 dan 30 untuk ujian tarik keluar. Besi waja saiz 12 mm, 16 mm dan 25 mm 1400 mm panjang digunakan sebagai tetulang konkrit gelagar 1500 mm x 150 mm x 200 mm gred 20 di dalam ujian lenturan. Keputusan ujian menunjukkan, berat besi waja menurun 0.13%, 0.07% dan 0.02%, ukurlilit menurun 0.13%, 0.07% dan 0.02% pada 8 bulan dan pada 12 bulan berat besi waja menurun 0.13%, 0.04% dan 0.03%, ukurlilit menurun 0.12%, 0.04% dan 0.03% pada besi waja saiz 12 mm, 16 mm and 25 mm untuk ujian regangan. Pada 8 bulan dan 12 bulan berat besi waja menurun 0.14%, 0.05% dan 0.02%, ukurlilit menurun 0.13%, 0.06% dan 0.02% pada besi waja saiz 12 mm, 16 mm and 25 mm untuk ujian tarik keluar pada konkrit ared 20 dan konkrit gred 30. Pada 8 bulan dan 12 bulan berat besi waja menurun 0.08%, 0.05% dan 0.01%, ukurlilit menurun 0.08%, 0.04% dan 0.01% pada besi waja saiz 12 mm, 16 mm dan 25 mm untuk ujian lenturan konkrit gred 20. Pada 4 bulan, berat dan ukurlilit besi waja bersaiz 12 mm, 16 mm dan 25 mm tidak berubah, digunakan di dalam ujian tarik keluar konkrit gred 20 dan konkrit gred 30, dan ujian lenturan konkrit gred 20. Kajian menunjukkan karatan yang tidak dibersihkan pada permukaan besi waja mempengaruhi kekuatan di dalam ikatan antara besi waja dan konkrit pada 4 bulan tetapi tidak mempengaruhi kekuatan dalam ikatan antara besi waja dan konkrit pada 8 bulan dan 12 bulan pengkaratan setelah karatan dibersihkan pada permukaan besi waja.



### **TABLE OF CONTENTS**

	Page
TITLE	i
DECLARATION	ii
CERTIFICATION	iii
ACKNOWLEDGEMENT	iv
ABSTRACT	v
ABSTRAK	vi
TABLE OF CONTENTS	vii
LIST OF TABLES	x
LIST OF FIGURES	xii
LIST OF SYMBOLS	xvi
LIST OF ABBREVIATIONS	xvii
LIST OF APPENDIX	xviii



CHAP	TER 1: INTRODUCTION	
1.1	Introduction	1
1.2	Problem Statement	4
1.3	Objectives	5
1.4	Significance of the Study	6
1.5	Scope of the Study	7
CHAF	TER 2: LITERATURE REVIEW	
2.1	Introduction	13
2.2	Corrosion Mechanism	14
2.2.1	How Steel Bars Corrode without Concrete	15
2.2.2	How Steel Bars Corrode with Concrete	15
2.3	Bond Mechanism	19
2.3.1	The Bond Strength between Steel Bars and Concrete	19
2.3.2	Factors Affecting the Bond Strength	23
2.4	Tensile Test	26
2.5	Pullout Test	40
2.6	Flexural Test	55
2.7	Summary	64
СНАР	TER 3: METHODOLOGY	66
3.1	Introduction	66
3.2	Research Flowchart	69
3.3	High Yield Steel Bars	70
3.4	Concrete	73
3.5	Description of Period and Specimen	78
3.5.1	Specimen for the Tensile Test	80
3.5.2	Specimen for the Pullout Test	81
3.5.3	Specimen for the Flexural Test	83
3.5.4	Specimen for the Slump Test	85
3.5.5	Specimen for the Compressive Test	86
3.6	Specifications of Testing Procedures	87
3.7	The Experimental Programs	89
3.7.1	Coarse Aggregates Sieve Test (Granite Stone)	90
3.7.2	Fine Aggregates Sieve Test (River Sand)	90
3.7.3	Slump Test	91
3.7.4	Compressive Test	92
3.7.5	Tensile Test	93
3.7.6	Pullout Test and Flexural Test	94



CHAP	PTER 4: RESULTS and DISCUSSION	102
4.1	Introduction	102
4.2	Preliminary Result	102
4.2.1	Coarse Aggregates and Fine Aggregates Sieve Result	102
4.2.2	Slump Test Result	108
4.2.3	Compressice Test Result	108
4.3	Discussion	109
4.3.1	Tensile Test	109
4.3.2	Pullout Test for Concrete Grade 20 and Grade 30	118
	and a Flexural Test for Concrete Grade 20	
CHAP	TER 5: CONCLUSION	147
5.1	Introduction	147
5.2	Conclusion	148
5.3	Recommendation	148
REFE	RENCES	149
APPE	NDIX	158



### LIST OF TABLES

#### Page

Table 2.1:	Steel Type, Nominal Size, Strength and Minimum Elongation	31
Table 2.2:	Malaysian Standard 146:2006 (Third Revision)	32
Table 3.1:	Specimen for the Tensile Test	80
Table 3.2:	Specimen for the Pullout Test for Concrete Grade 20	81
Table 3.3:	Specimen for the Pullout Test for Concrete Grade 30	82
Table 3.4:	Specimen for the Flexural Test for Concrete Grade 20	83
Table 3.5:	Slump Test for the Pullout Test and Flexural Test Concrete Concrete Grade 20	85
Table 3.6:	Slump Test for the Pullout Test Concrete Grade 30	85
Table 3.7:	Compressive Test for the Pullout Test and Flexural Test for Concrete Grade 20	86
Table 3.8:	Compressive Test and Pullout Test for Concrete Grade 30	86
Table 3.9:	Slump Limit for the Fresh Concrete Grade 20 and Grade 30	91
Table 3.10:	Concrete Grade 20 and Grade 30	92
Table 3.11:	Compressive Strength for Concrete Grade 20 and Grade 30	93
Table 3.12:	High Yield Steel Bars Periods of Corrosion for the Tensile Test	94
Table 3.13:	High Yield Steel Bars Periods of Corrosion for Pullout Test for Concrete Grade 20 and Grade 30	97
Table 3.14:	High Yield Steel Bars Periods of Corrosion for Flexural Test for Concrete Grade 20	99



Table 4.1:	Shows the Size Distribution of Granite Stone Sieve Test Result	106
Table 4.2:	Shows the Size Distribution of River Sand Sieve Test Result	107
Table 4.3:	Slump Test Result for Concrete Grade 20 and Grade 30	108
Table 4.4:	Compressive Test Result for Concrete Grade 20 and Grade 30	108
Table 4.5:	Weight of High Yield Steel Bars	113
Table 4.6:	Area of High Yield Steel Bars	114
Table 4.7:	Tensile Test Result of High Yield Steel Bars	115
Table 4.8:	Weight of High Yield Steel Bars for Concrete Grade 20 and Concrete Grade 30	121
Table 4.9:	Area of High Yield Steel Bars for Concrete Grade 20 and Concrete Grade 30	122
Table 4.10:	Pullout Test Result for Concrete Grade 20 and Concrete Grade 30	123
Table 4.11:	Weight of High Yield Steel Bars for Concrete Grade 30	138
Table 4.12:	Area of High Yield Steel Bars for Concrete Grade 20	139
Table 4.13:	Weight of Mild Steel Bars for Concrete Grade 20	140
Table 4.14:	Area of Mild Steel Bars for Concrete Grade 20	141
Table 4.15:	Flexural Test Result for Concrete Grade 20	142



### LIST OF FIGURES

Figure 1.1:	Corroded High Yield Steel Bars laid on the Ground	2
Figure 1.2:	Corroded Reinforcement	2
Figure 2.1:	Schematic Representation of the Electrochemical Process of the Chloride-Induced Pitting Corrosion	18
Figure 2.2	Local Bond Stress-Slip Law (FIB 2000)	20
Figure 2.3:	Flow Stress Curve with Yield and Ultimate Tensile Strength	30
Figure 2.4:	Stress-Strain Curve	30
Figure 2.5:	Axial Tensile Load	34
Figure 2.6:	Stress-Strain Graph	36
Figure 2.7:	Typical Morphology of Specimen for Assessment of Bond-Slip Law where Concrete is Partly Stress in Compression (a) and (b) Setup of Direct Tension Pullout Bond Test where the Whole System is Stressed in Tension (c)	43
Figure 2.8:	View of the Tensile Ring (R. Tepfers, 1973)	44
Figure 2.9:	Internal Cracks around the Reinforced Bar Embedded in Concrete (Y. Goto, 1971)	45
Figure 2.10:	Flexural Test Graph	58
Figure 3.1:	Flowchart Methodology	69
Figure 3.2:	High Yield Steel Bars 600 mm Long exposed to Weathers for the Tensile Test	70
Figure 3.3:	High Yield Steel Bars 800 mm Long exposed to Weathers for the Pullout Test	71



۲

Figure 3.4:	High Yield Steel Bars 1400 mm Long tied with 100 mm X 150 mm X 100 mm X 150 mm Stirrups exposed to weathers for the Flexural Test	71
Figure 3.5:	High Yield Steel Bars 800 mm Long and Concrete Cubes Mold 150 mm X 150 mm X 150 mm used for the Pullout Test	72
Figure 3.6:	High Yield Steel Bars 1400 mm Long tied with 100 mm X 150 mm X 100 mm X 150 mm Stirrups and 1500 mm X 150 mm X 200 mm Wooden Formwork for the Flexural Test	72
Figure 3.7:	Universal Testing Machine, Digital Servo Control (IPC) used for the Tensile Test	73
Figure 3.8:	Cubes 150 mm X 150 mm X 150 mm for the Compressive Test	74
Figure 3.9:	Reinforced-Concrete Cubes 150 mm X 150 mm X 150 mm for the Pullout Test	74
Figure 3.10:	Reinforced-Concrete Beams 1500 mm X 150 mm X 200 mm for the Flexural Test	75
Figure 3.11:	Apparatus used to Sieve Coarse Aggregates (Granite Stone)	75
Figure 3.12:	Apparatus used to Sieve Fine Aggregates (River Sand)	76
Figure 3.13:	Apparatus used for Slump Test and Concrete Cubes Mold for Concrete Cubes 150 mm X 150 mm X 150 mm	76
Figure 3.14:	ADR 1500, Machine used for the Compressive Test	77
Figure 3.15:	Shimadzu UH 300kNI, Machine used for the Pullout Test	77
Figure 3.16:	Shimadzu 1000kN, Machine used for the Flexural Test	78
Figure 4.1:	Shows the Granite Stone Sieve Test Result Inside Enveloped (BS 410 2:2000)	106



Figure 4.2:	Shows the River Sand Sieve Test Result Inside Enveloped (BS 410 1:1000)	107
Figure 4.3:	Shows 12 mm, 16 mm and 25 mm Diameter 600 mm Long High Yield Steel Bars Weight and Area	116
Figure 4.4:	Shows 12 mm, 16 mm and 25 mm Diameter 600 mm Long High Yield Steel Bars Breaking and Elongation Percentage	116
Figure 4.5:	Tensile Test used UTM 1000 Universal Machine	117
Figure 4.6:	High Yield Steel Bars Split into Two-Halves	117
Figure 4.7:	Shows 12 mm, 16 mm and 25 mm Diameter 800 mm Long High Yield Steel Bars Weight and Area	124
Figure 4.8:	Shows the Pullout Test Result in Concrete Grade 20	124
Figure 4.9:	Shows the Pullout Test Result in Concrete Grade 30	125
Figure 4.10:	Shows Concrete Cover for 12 mm Diameter for Concrete Grade 20 and Grade 30	125
Figure 4.11:	Shows Concrete Cover for 16 mm Diameter for Concrete Grade 20 and Grade 30	126
Figure 4.12:	Shows Concrete Cover for 25 mm Diameter for Concrete Grade 20 and Grade 30	126
Figure 4.13:	Bond-Stress Generated on the High Yield Steel Bar	127
Figure 4.14:	Force Parallel and Perpendicular of the High Yield Steel Bar Concrete Interface	127
Figure 4.15:	Concrete Cube Slipped from High Yield Steel Bar	128
Figure 4.16:	High Yield Steel Bar Removed from the Concrete Cube	128
Figure 4.17:	Concrete Cubes Pull Out of the 12 mm ø High Yield Steel Bars	129
Figure 4.18:	Concrete Cubes Pull Out of the 16 mm ø High Yield Steel Bars	129



Figure 4.19:	Concrete Cubes Pull Out of the 25 mm ø High Yield Steel Bars	130
Figure 4.20:	Concrete Cubes Pull Out of the 12 mm ø, 16 mm ø and 25 mm ø High Yield Steel Bars	130
Figure 4.21:	Pullout Test used Shimadzu 300kNI Machine	131
Figure 4.22:	Shows 12 mm, 16 mm and 25 mm Diameter 1400mm Long High Yield Steel Bars Weight and Area	143
Figure 4.23:	Shows 8 mm Diameter Mild Steel Bar Stirrups 100 mm x 150 mm x 100 mm x 150 mm Weight and Area	143
Figure 4.24:	Shows the Flexural Test Result in Concrete Grade 20	144
Figure 4.25:	Flexural Test for Reinforced-Concrete Beam embedded with 12 mm Diameter High Yield Steel Bars used Shimadzu 1000kN Machine	144
Figure 4.26:	Flexural Test for Reinforced-Concrete Beam embedded with 16 mm Diameter High Yield Steel Bars used Shimadzu 1000kN Machine	145
Figure 4.27:	Flexural Test for Reinforced-Concrete Beam embedded with 25 mm Diameter High Yield Steel Bars used Shimadzu 1000kN Machine	145
Figure 4.28:	Features of Uncleaned High Yield Steel Bars on the Four Months of Corrosion	146
Figure 4.29:	Features of Cleaned High Yield Steel Bars on the Eight Months and 12 Months of Corrosion	146



### LIST OF SYMBOLS

- - Degree
- Ø Diameter
- litres
- % Percentage
- ± Plus or Minus
- ε Strain
- σ Stress
- µm micrometer or micron



### LIST OF ABBREVIATIONS

ACI	-	American Concrete Institute
ASTM	-	American Society for Testing and Materials
BS	-	British Standard
MS	-	Malaysian Standard
N/mm²	-	Newton millimetre square
N	-	Newton
mN	-	milliNewton
kN	-	KiloNewton
MN	-	MegaNewton
Mm	-	millimetre
mm²	-	millimetre square
mm³	-	millimetre cubic
mm/s	-	millimetre per second
m²	-	meter square
m³	-	meter cubic
kg	-	Kilogram
ft²	-	feet square
ft³	-	feet cubic
F	-	Fahrenheit
С	-	Celsius
hy	-	high yield
hysb	-	high yield steel bar



### LIST OF APPENDIX

Page

Appendix	Map and Locations of Specimens Production, Testing and to	158
	Obtain Results	



### **CHAPTER 1**

#### INTRODUCTION

#### 1.1 Introduction

One of the most significant currents of discussions is regarding the corrosion of the high yield steel bars. Corrosion of high yield steel bars are commons problem faced by contractors and constructors, and becoming increasingly difficult and sometimes been ignored in practices of keeping the material in the construction site. High yield steel bars are common material characterized by iron and an important component that reinforces the concrete for better and stronger bond strength. The corrosion of the high yield steel bars becomes an issue in a structural buildings industry for past developments. Corrosion of high yield steel bars has been thoughts of the factors in giving bad bond strength to reinforces the concrete, and which is most widely used in a constructions development industry. Corrosion of high yield steel bars occurs because it is placed on the ground and expose to weathers for the long periods of time, presented in Figure 1.1 and Figure 1.2 respectively.





Figure 1.1: Corroded High Yield Steel Bars laid on the Ground



Figure 1.2: Corroded Reinforcement



High yield steel bars, one of the most common and useful materials used to reinforces the concrete in the structural constructions, and it has one major flaw, it corrodes when exposed to oxygen and water. High yield steel bars are most highly manufactured by man-made materials and its subject to corrosion as it is was made almost entirely of iron in its production respectively.

Mechanism of the corrosion occurs when iron (Fe) surface undergoes simple changes as following:

- a. First, (Fe → Fe<sup>n+</sup> + n electrons) iron atoms lose some of the electrons and become a positively charged ion, and allow it to bond with another group of the negatively charged atom. The negatively charged atoms combined with electrons and the specimens (high yield steel bars) produce the first rusting reaction (Malaysia Steel Industry).
- b. Second, the reaction involves oxygen (O₂) and water (H₂O) or (O₂ + 2H₂O + 4e<sup>-</sup> → 4OH<sup>-</sup>) to give a variant of iron oxide to rusted the wet iron (Malaysian Steel Industry).

 $2Fe + O_2 + 2H_2O \rightarrow 2Fe(OH)_2$ 

Iron (High Yield Steel Bars) + Oxygen + Water  $\rightarrow$  Iron Hydroxide dissolved in it. Oxygen dissolves in water and reacts with Iron Hydroxide caused Corrode.  $4Fe(OH)_2 + O_2 \rightarrow 2H_2O + 2Fe_2O_3H_2O$ Iron Hydroxide + Oxygen  $\rightarrow$  Water + Hydrated Iron Oxide (Brown color corrode). (Malaysian Steel Industry).



In this study, high yield steel bars reinforced concrete cubes 150 mm X 150 mm X 150 mm dependent on bar's sizes, concrete covers, and position of high yield steel bars. Its mechanism allowed the anchorage positions and composites actioned which reacts when concrete cube pulled out from high yield steel bar, which causes slips and cracks during the process to separates both of the specimens. High yield steel bars reinforce the concrete beams 1500 mm X 150 mm X 200 mm dependent on bar's sizes, concrete covers, spacing and positions between high yield steel bars and stirrups. Its mechanism allowed the anchorages of the horizontal, parallel and vertical positions of high yield steel bars reinforce for its bond strength, and it also allowed composites action reacts when the loads applied to the concrete beam during flexing respectively.

#### 1.2 Problem Statement

It has been discussed regarding corrosion of the high yield steel bars by contractors and constructors that confused and uncertainty, how corrosion can affects the bond strength between corroded high yield steel bars and concrete. However, there are little attentions has been paid to prevents corrosion for the unstable practicals ways of keeping high yield steel bars. In Sandakan, most of high yield steel bars that are used in the construction abandoned for months and hence, the high yield steel bars exposures to weathers pore to the corrosion and moreover, contractors also tend to purchases corroded steel bars without knowing the actuals strength and therefore, the researcher focuses on the tensile strength and the bond strength respectively.



The critical of the study is to investigate and reveal the finding on high yield steel bars strength on its tensile strength and bond strength between the high yield steel bars and concrete for pullout strength and flexural strength of uncorroded and corroded high yield steel bars which corrosion is uncleaned and cleaned on surfaces of high yield steel bars might be. The limitation of this production for the specimens are all high yield steel bars are new and come in a bundle of each size ranges from 8 mm ø, 12 mm ø, 16 mm ø and 25 mm ø. The brand new high yield steel bars cut 600 mm length of 12 mm ø, 16 mm ø and 25 mm ø, 800 mm length of 12 mm ø, 16 mm ø and 25 mm ø. The brand new high yield steel bars cut 500 mm length of 8 mm ø mild steel for stirrups of 100 mm X 150 mm X 100 mm X 150 mm which tie with the 1400 mm long high yield steel bars then kept above the ground level of 76 cm under the shed for corrosion process respectively.

#### 1.3 Objectives

The aimed of this study is to investigate the tensile strength and the bond strength of the high yield steel bars that subjected to different periods of corrosion. In order to achieve the aims the following objectives are set:

**Objective 1:** To investigates high yield steel bars that exposed to weathers for 4 months, 8 months and 12 months of corrosion for the tensile strength.

**Objective 2:** To investigates high yield steel bars that exposed to weathers for 4 months, 8 months and 12 months corrosion, high yield steel bars embedded vertically in concrete cubes 150 mm X 150 mm X 150 mm for the pullout test.



#### REFERENCES

Abrishami, H.H. Mitchell, D., 1992. Simulation of uniform bond stress. ACI Mat. JI 89(2): 161-168.

Adukpo, E. Oteng-Seifah, S. Manu, P. and Solomon-Ayeh K., 2010. The effect of steel reinforcement corrosion on tensile strength, bond strength and flexural strength. In: Barrett, P., Amaratunga, D., Haigh, R., Keraminiyage, K. and Pathirage, C., 2010. Eds. CIB 2010 World Congress, Salford, UK: 166-175.

Alengaram, UJ, Mahmud, H. and Jumaat, MZ, 2010. Comparison of mechanical and bond properties of oil palm kernel shell concrete with normal weight concrete, International Journal of the Physical Sciences 5(8): 1231-1239.

Alk. Apostolopoulos, T. Matikas, C.A. Apostolopoulos, G. Diamantogiannis, 2013. Pit corrosion examination of bare and embedded steel bar, in: 10th International Conference Advanced Metallic Materials and Technology, Saint Petersburg, Russia: 489-495.

Almusallam A.A., 2001. Effect of degree of corrosion on the properties of reinforcing steel bars, Construction and Building Materials, 15(8): 361-8.

Assaad, JJ & Issa, CA., 2012. Bond strength of epoxy-coated bars in underwater concrete, Construction and Building Materials, 30: 667-674.

Azizinamini, A. Pavel, R. Hatfield, E. and Ghosh, S.K., 1999. Behavior of spliced reinforcing bars embedded in high strength concrete. ACI Str JI 96(5): 826-835.

Azizinamini, A. Darwin, D. Eligehausen, R. Pavel, R. and Ghosh, S.K., 1999. Proposed modification to ACI 318-95 tension development and lap splice for high strength concrete. ACI Str JI 96(6): 922-926.

Azizinamini, A.Stark, M. Roller, J.J. and Ghosk, S.K., 1993. Bond performance of reinforcing bars embedded in high strength concrete. ACI Str JI 90(5): 554-561.

American Concrete Institute (ACI) Committee 365. 2002. Service life prediction state of-art report. ACI 365. 1R-00 American Concrete Institute, Farmington Hills, MI, 44.



ASTM A615-16: Standard Specification for Deformed and Plain Carbon-Steel Bars for Concrete Reinforcement. West Conshohocken: ASTM International, 2016.

Bonacci J. and Marquez J., 1994. Tests of Yielding Anchorages under Monotonic Loadings. Journal of Structural Engineering 120(3): 987-997.

Broomfield and J., 1997. Corrosion of steel in concrete: understanding, investigating and repair, E&FN Spon, London, 264.

Cairns J., 1979. An Analysis of the Ultimate Strength of Lapped Joints of Compression Reinforcement. Magazine of Concrete Research 31(106): 19-27.

Cao, J & Chung, DDL., 2001. Degradation of the bond between concrete and steel under cyclic shear loading monitored by contact electrical resistance measurement, Cement and Concrete Research 31(4): 669-671.

C.A. Apostolopoulos, S. Demis, V.G. Papadaksi, 2013. Chloride-include corrosion of steel reinforcement-mechanical performance and pit depth analysis, Constr. Build. Mater. 38(9): 139-146.

C.A. Apostolopoulos and V.G. Papadakis, 2007. Consequences of steel corrosion on the ductility properties of reinforcement bar. Construction and Building Materials 22(2008): 2316-24.

Chana and P., 1990. A Test Method to Establish a Realistic Bond Stress. Magazine of Concrete Research 24(151): 83-90.

Chen, G., Hadi, M.N.S., Gao, D. and Zhao, L., 2015. Experimental study on the properties of corroded steel fibres. Construction and Building Materials 79: 165-172.

Coronelli D. and Gambarova P., 2004. Structural assessment of corroded reinforced concrete beams. Modelling guidelines JI Struct Eng ASCE. 130(8): 1214-24.

Craig, R.R., ed. Mechanics of Materials. 1996, John Wiley & Sons: New York, NY, ISBN 0-471-50284-7



CEB-FIP Report., 2000. Bond of reinforcement in concrete: state of the art report. FIB Bulletin 10, Sw.

Darwin, D. Zuo, J. Tholen, M.L. and Idun, E.K., 1996. Development length criteria for conventional and high relative rib area reinforcing bars. ACI Str JI 93(3): 347-359.

Dowling N. E, 1993. Mechanical Behaviour of Materials, Engineering Methods for Deformation, Fracture and Fatigue, Fourth Edition., Marcia J. H., Holly S., Scott D., Somnath B., Ashwitha J., Jayashree A. and Daniel S., Pearson Education Limited 2013, Edinburgh Gate, Harlow, Essex CM20 2JE, England: 190-225.

Eligehausen, R. Popov, E.G. Bertero, V.V., 1983. Local bond stress-slip relationships of deformed bars under generalized excitations. R.No.UCB/EERC 83/23,EERC, Berkeley.

Fang, C, Lundgren, K, Chen, L and Zhu, Ch., 2004. Corrosion influence on bond in reinforced concrete. Cement and Concrete Research 34(11): 2159-2167.

Ferguson, P.M. Breen, J.E. Thompson, J.N., 1966. Pull out tests on high strength reinforcing bars. ACI JI T.No.62-55: 933-950.

Ferguson, P.M. Robert, I. Thompson J.N., 1962. Development length of high strength reinforcing bars in bond. ACI JI T.No.59-17: 887-922.

Fernandez et al., 2016. Mechanical model to evaluate steel reinforcement corrosion effects on  $\sigma$ - $\epsilon$  and fatigue curve. Experimental calibration and validation 118: 320-333.

Francois, R., Khan, I. & Dang V.H., Mater Struct., 2013. Impact of corrosion on mechanical properties of steel embedded in 27-year-old corroded reinforced concrete beams. 46:899. doi: 10.1617/s11527-012-9941-z.

Foroughi, A, Dilmaghani, S and Famili, H., 2008. Bond reinforcement steel in selfcompacting concrete, International Journal of Civil Engineering 6(1): 24-33.

Gambarova P.G., Rosati G.P., Zasso B., 1989. Steel-to-Concrete Bond after Concrete Splitting. Test Results. Materials and Structures 22: 35-47.



Goto and Y., 1971. Cracks formed in concrete around deformed bars in concrete. ACI JI: 68(2): 244-251.

H. Kim, S. Tae, H. Lee, S. Lee, T Noguchi, 2009. Evaluation of mechanical performance of corroded reinforcement considering the surface shape. ISIJ Int. 49(9): 1392-1400.

H.S. Lee and Y.S. Cho, 2009. Evaluation of the mechanical properties of steel reinforcement embedded in concrete specimen as a function of the degree of reinforcement corrosion, Int. J. Fract. 157(1-2): 81-88.

H.S. Lee, F. Tomosawa, T. Noguchi, 1996. Effect of rebar corrosion on the structural performance of single reinforced beams C. Sjostrom (Ed.), Durability of Building Materials and Components vol. 1, E & FN Spon, London: 571-580.

Hadi, MNS, 2008. Bond of high strength concrete with high strength reinforcing steel. The Open Civil JI 2: 143-147.

Hanjari K.Z., Kettil P., Lundgren K., 2011. Analysis of mechanical behaviour of corroded reinforced concrete structures. ACI Struct. JI 108(5): 532-41.

Hansen, R.J. and Liepins, A.A., 1962. Behaviour of bond in dynamic loading. ACI JI: 563-583.

Harajli, M.H. Hamad, B.S. and Rteil, A.A., 2004. Effect of confinement on bond strength between steel bars and concrete. ACI Struct. JI 101(5): 595-603.

Higgins C. and Farrow III W.C., 2006. Tests of reinforced concrete beams with corrosion damaged stirrups. ACI Struct. Jl 103(1): 133-41.

#### https://en.wikipedia.org/wiki/Flexural-strength

https://www.google.com/imgres=httpFimage.thefabricator.com

https://theconstructor.orgcivilengineering/checkforfixingreinforcementinformworksofco ncretestructuralmembers



https://www.researchgate.net/figure/local-bond-stress-slip-law-FIB-2000\_fig8\_286916341

https://www.researchgate.net/figure/schematic-representation-of-the-electrochemical-process-of-chloride-induced-pitting-fig1\_319664970

https://www.thefabricator.com/article/matelsmaterials/determining-the-flowstresscurve-with-yield-and-ultimate-tensile-strength-part-i

https://www.thewarren.org/AlevelRevision/engineering/stress-strain

IS 2770., 1997. Method of Testing bond in reinforced concrete part i-pullout test. BIS, New Delhi.

J. Cairns, G.A. Plizzari, Y. Du, D.W. Law, C. Franoni, 2005. Mechanical properties of corrosion-damaged reinforcement, ACI Mater. Jl 102(4): 256-264.

J. Xia, W. Jin, Y. Zhao, L. Li, 2013. Mechanical performance of corroded steel bars in concrete. Struct. Build. 166(5): 235-246.

Johnson, JB., 2010. Bond Strength of Corrosion Resistant Steel Reinforcement in Concrete, MSc Thesis, Faculty of the Virginia Polytechnic Institute and State University.

L.A. Clark and M. Saifullah., 1994. Effect of corrosion rate on the bond strength of corroded reinforcement, in: R.N. Swamy (Ed.), Corrosion and Corrosion Protection of Steel in Concrete, Sheffield Academic Press, Sheffield, United Kingdom.

Larrard F., Schaller I., Fuchs J., 1993. Effect of Bar Diameter on the Bond Strength of Passive Reinforcement in High-Strength Concrete. ACI Materials JI 90(4): 333-339.

Lutz L.A. and Gergely, P., 1967. Mechanics of bond and slip of deformed bars in concrete. ACI Mat. JI T.No.64-62: 711-721.

M. Maslehuddin, I.M. Allam, G. Al-Sulimani, A.I. Al-mana, S.N. Abduljauward., 1990. Effect of rusting of reinforcing steel on its mechanical properties and bond with concrete. ACI Mater. JI 87(5): 496-502.





M. Papadopoulos et. al., 2011. Corrosion of exposed rebars, associated mechanical degradation and correlations with accelerated corrosion tests. Constr. Build. Mater. 25(8): 3367-3374.

Malvar L.J., 1992. Bond of Reinforcement under Controlled Confinement. ACI Materials JI 89(6): 593-601.

Mathey, R.G. Watstein, D., 1961. Investigation of bond in beam and pull out specimens with high yield strength deformed bars. ACI JI T.No.57-50: 1071 1089.

M.M. Kashani, A.J. Crewe, N.A. Alexander, 2013. Nonlinear stress-strain behavior of corrosion-damaged reinforcing bars including inelastic buckling. Engineering Struct. 48(3): 417-429.

Moetaz, M and El-Hawary, 1999. Evaluation of bond strength of epoxy-coated bars in concrete exposed to marine environment. Construction and Building Materials 13(7): 357-362.

Nihal Abdelhamid Taha and Mohammed Morsy, 2015. Study of the behavior of corroded steel bar and convenient method of repairing. HBRC JI 2016. 12(2): 107-113.

O'Flaherty F.J., Mangat P.S., Lambert P., Browne E.H., 2008. Effect of underreinforcement on the flexural behaviour of corroded beams, Material Struct. 41(2): 311-21.

R. Palsson and M.S. Mirza, 2002. Mechanical response of corroded steel reinforcement of abandoned concrete bridge, ACI Struct. JI 99(2): 157-162.

S. Morinaga, 1996. Remaining life of reinforced concrete structures after corrosion cracking, in: C. Sjostrom (Ed.), Durability of Building Materials and Components. E & FN Spon, London, United Kingdom.

Schiessl, P., 1988. Corrosion of steel in concrete. Report of the TC60-CSC RILEM, Chapman and Hal, London: 204.

Selvaraj, R & Bhuvaneshwari, B., 2009. Characterization and development of organic coating for steel rebars in concrete. Electrochimica Acta 27(6): 657 670.





Somayaji, S. and Shah, S.P., 1981. Bond stress versus slip relationship and cracking response of tension members. ACI JI 78(3): 217-225.

Soylev, TA & Francois, R., 2006. Effects of bar-placement conditions on steel concrete. Materials and Structures 39(2): 187-195.

Soroushian, P. Choi, K.B. Park, G.H. and Aslani, F., 1991. Bond of def. bars to concrete: effects of confinement and strength of concrete. ACI Mat. JI 88(03): 227-232.

Soroushian, P. Choi and K.B. 1989. Local bond of deformed bars with different diameters in confined concrete. ACI Str JI 86(02): 217-222.

Stewart M.G., 2009. Mechanical behaviour of pitting corrosion of flexural and shear reinforcement and its effect on structural reliability of corroding RC beams. Struct Safety 31(1): 19-30.

Tepfers R., 1979. Cracking of Concrete Cover along Anchored Deformed Reinforcing Bars. Magazine of Concrete Research 31 (106): 3-12.

Tepfers, R.A. 1973. Theory of bond applied to overlapped tensile reinforcement splices for deformed bars. Publ 73:2. Department of Concrete Structures, Chalmers University of Technology, Göteborg. 328.

Torres-Acosta A.A., Navarro-Gutierrez S., Teran-Guillen J., 2007. Residual flexure capacity of corroded reinforced concrete beams, Engineering Struct. 29(6): 1145-52.

Tuutti K., 1982. Corrosion of steel in concrete, Report 4, Swedish Cement and Concrete Research Institute, Stockholm, Sweden. P 82.

Val D.V., 2007. Deterioration of strength of RC beams due to corrosion and its influence on beam reliability, Structural Eng. JI 133(9): 1297-306.

Valcuende, M & Parra C, 2009. Bond behaviour of reinforcement in self compacting concretes. Construction and Building Materials 23(1): 162-170.



Verma, N & Balasubramaniam R, 2011. Corrosion of Steel Reinforcements in Concrete. MME 480 TERM PAPER, Indian Institute of Technology, Kanpur.

Vidal T., Castel A., Francois R., 2007. Corrosion process and structural performance of a 17 year old reinforced concrete beam stored in chloride environment, Cem Concr Res. 37(11): 1551-61.

W. Zhang, X. Song, X. Gu, S. Li., 2012. Tensile and fatigue behavior of corroded rebars, Constr. Building Mater. 34(9): 409-417.

Warner, RF, Rangan, BV, Hall, AS & Faulkes, KA., 1998. Structures. Longman Australia.

Yalciner, H, Eren, O & Sensoy S, 2012. An experimental study on the bond strength between reinforcement bars and concrete as a function of concrete cover, strength and corrosion level. Cement and Concrete Research 42(5): 643-655.

Yankelevsky, D.Z. Adin, M.A. and Farhey, D.N., 1992. Mathematical model for bond slip behaviour under cyclic loading. ACI Str JI 89(6): 692-698.

Yu Chen Ou and Nguyen Dang Nguyen, 2016. Modified axial-shear-flexure interaction approaches for uncorroded and corroded reinforced concrete beams. Engineering Structure 128: 44-54.

Yu Chen Ou, Yudas Tadeus, Teddy Susanto and Hwasung Roh, 2015. Tensile behavior of naturally and artificially corroded steel bars. Construction and Building Materials 103 (2016): 93-104.

Y. Du, L.A Clark, A.H.C. Chan, 2005. Residual capacity of corroded reinforcing bars, Mag. Concr. Res 57(3): 135-147.

Y. Du, L.A Clark, A.H.C. Chan, 2005. Effect of corrosion on ductility of reinforcing bars, Mag. Concr. Res 57(7): 407-419.

Y. Du, 2001. Effect of reinforcement corrosion on structural concrete ductility (Ph.D. dissertation), University of Birmingham, United Kingdom.



Y.G. Du, L.A. Clark, A.H.C. Chan, 2005. Residual capacity of corroded reinforcing bars Mag. Concr. Res. 57(3): 135-147.

Y.S. Yuan, Y.S. Ji, S.P. Shah, 2007. Comparison of two accelerated corrosion techniques for concrete structures ACI Struct. JI 104(3): 344-347.

Zhang, R., Castel, A., Francois R., 2009. Serviceability limit state criteria based on steel concrete bond loss for corroded reinforced concrete in chloride environment, Materials Structure 42(10): 1407-21.

